HANDBOOK OF FIREPROOF CONSTRUCTION



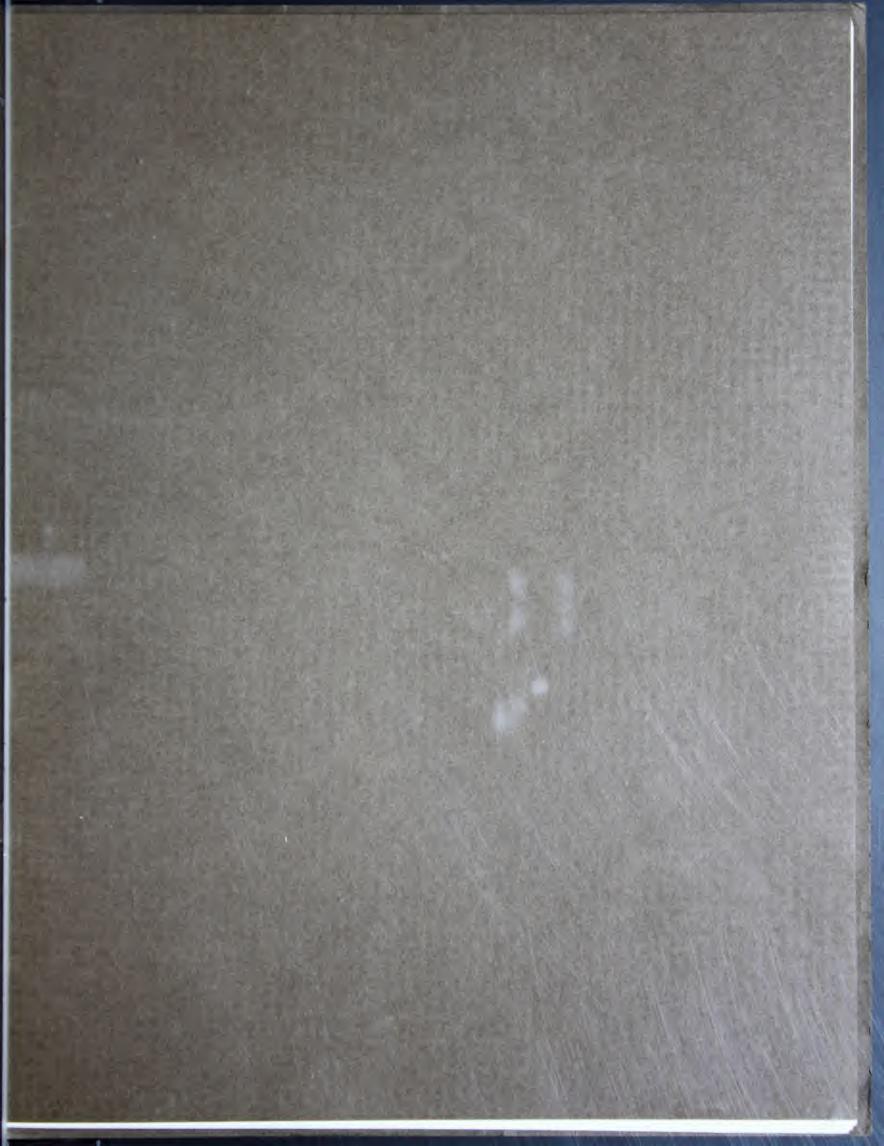


MEYER STEELFORM CONSTRUCTION

CONCRETE ENGINEERING CO. OMAHA.

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CONCRETE ENGINEERING CO





HANDBOOK * of * FIREPROOF CONSTRUCTION

This Handbook is issued to familiarize the engineer, architect and builder with the many advantages of Meyer Steelform Construction and Ceco Fireproofing Materials.

Meyer Steelform Construction is a standard system of concrete floor construction in present day use by prominent architects, engineers and contractors in important structures throughout the country.

Ceco Fireproofing Materials are manufactured in styles approved by the leading architects and engineers. Utmost economy and quality in both materials and the subsequent erection has been the thought foremost in the minds of the designers.

This Handbook is therefore in every respect a Handbook of Fireproof Construction. We have endeavored to make it complete, explaining in detail the economy, efficiency, accuracy and adaptability of Meyer Steelform Construction and Ceco Fireproofing Materials.

Your attention is invited to the co-operative service maintained by our Engineering and Contract Departments and the opportunity to serve you is solicited.

CONCRETE ENGINEERING CO.

This sheet conforms to the size for advertising matter adopted by the American Institute of Architects

"MAXIMUM ENGINEERING SERVICE"

To render service is to do for your fellow man all that you would do for yourself in similar circumstances, with all possible enthusiasm, honor and efficiency. It is like the building of a monument that will forever withstand the test of time.

It is possible to render such service in the designing and erection of the modern fireproof structure, that the individual interests of architect, owner and contractor are each served to the greatest extent.

This is "Maximum Engineering Service."







MEYER STEELFORM CONSTRUCTION

PATENTED

INTRODUCTION

THE development and use of reinforced concrete in every type of fireproof structure has been so rapid and extensive, and its many advantages are so generally recognized, that it requires no detailed description or explanation. It will suffice to say that no other building material, or combination of materials, possesses the advantages of low initial and maintenance costs, quick erection, architectural adaptability, fireproofness and permanence, in the same degree as reinforced concrete.

There are various types of concrete design which have been adopted as standard, all of which admirably serve their purpose. It is the function of this Handbook to present to the reader the superior features of economy and quality found in the design of the reinforced concrete joist floor and in Meyer Steelform Construction.

The economy of the reinforced concrete joist floor is easily understood by comparing it with the wood joist floor. For ordinary conditions of loading, no type of wood floor has ever been developed which is more economical than the wood joist floor. The same economy of materials and labor is found in the design of reinforced concrete floors using joists at stated intervals for carrying the load. Its economy is more apparent in the longer spans, and in such structures as schools, apartments, hotels, office buildings, warehouses, garages, store buildings, etc., it is by far the most satisfactory and economical. Less concrete and steel are required to carry the given load with this type of floor design, and with the deep ribs or joists of concrete tying the structure together it is very desirable for buildings having moving or vibrating loads.

After this economy in materials required for the reinforced concrete joist floor had been well established, the next development toward economy occurred in the form work. Considering the fact that the average cost of the form work is approximately one third of the total cost of the concrete work, and that there is but little salvage in a large portion of the form work after the completion of the building, it is the most logical place to introduce economy. Economy in form work is in exact ratio with the re-use obtained, and it is now generally acknowledged that the use of a permanent equipment of removable steel forms in standard sizes effects the greatest possible economy.

It is therefore evident, that the economy of Meyer Steelform Construction lies in the minimum of materials required for the concrete joist floor, and in the removal and reuse features of Meyer removable Steelforms.





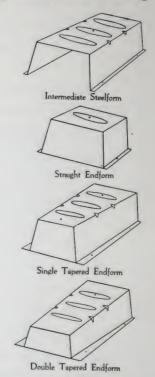
DESCRIPTION

In building the form work for the concrete joist floor, Meyer Steelforms are used as a mold for the joists and the intervening slab, the load being carried by the joists in one direction to the supports. Continuous joists are produced by lapping the Steelforms, and the ends of the rows of Steelforms are closed with Endforms.

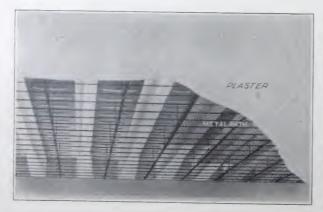
INTERMEDIATE STEELFORMS have depressed ribs in the top surface, thus securing the necessary rigidity to withstand the heavy trucking loads and weights which occur during construction. Being

made of 16 gauge sheet steel, and formed into exact shape by heavy presses, they are absolutely rigid. The lower flanges are provided with nail holes so that the Steelforms can be accurately and firmly placed in position on the centering. 3-16" round openings are placed in the center of the top surface of each Intermediate Steelform to permit placing ceiling hangers in the slab above each Steelform. (See details on page 7.)

Endforms are of three types—Straight, Single Tapered and Double Tapered. They are all used to close the rows of Intermediate Steelforms, the Straight Endforms being used only where the load is such that the same width of joist can be maintained throughout the span between supports. Single Tapered Endforms effect an increase in the width of the joist as it approaches the support, thus providing the necessary concrete where the shear is greatest and where it is needed for negative compression at the support. Double Tapered Endforms, in addition to increasing the width of the joist, also increase the depth of the compression flange or tee of the supporting beam or girder, without increasing the area of concrete below the neutral axis, where it is useless. The supporting flanges of both the Single Tapered and Double Tapered Endforms, are of the same width as the flanges of the Intermediate Steelforms, so that it is not necessary to provide outros contrains to take out of the same width as



vide extra centering to take care of the increase in the width of the joists. Tapered Endforms are very effective in producing economy in connection with long spans and heavy loads, strengthening the construction very effectively in shear and negative compression. They are an economical feature obtained only through the use of Meyer Steelform Construction. (See details on page 7.)



CEILING CONSTRUCTION: Our lath ceiling constructions involve the use of galvanized wire hangers, channel iron, ½" round steel pencil rods and lath. Complete details and specifications are shown on pages 7 and 8. Reference to these details will convince the reader of the great strength and permanency of the standard ceiling constructions. The weight of several men may easily be sustained by the carrying or furring channels. The ceilings, erected after the removal of the Steelforms, may be attached directly to the concrete joists or suspended.

When plastered, this effects the very desirable, hollow, soundproof floor, the air chambers between the joists and ceiling making a perfect insulation.





ADVANTAGES

Accuracy: Meyer Steelforms are so solidly rigid that absolute accuracy of concrete work is assured. Clean cut concrete joists are a certainty, and the lath ceiling is often omitted. The open concrete joist ceilings thus effected are very economical and satisfactory for use in Garages, Warehouses, Loft Buildings, etc. Reinforcing bars are securely held in their proper position by the use of Ceco Bar Chairs. There is no sagging of bars and no danger of incorrect placing of reinforcement. The erection of the ceiling construction after the removal of the Steelforms permits a thorough inspection of the concrete work before plastering.

EFFICIENCY: The solid rigidity of Meyer Steelforms permits their early removal and speedy re-use in the succeeding floors of any building. Sufficient equipment is furnished to the job to maintain the desired



speed of erection. The Steelforms are removed and re-used as the contractor knocks down and erects his wood form work beneath, temporary shores being erected at the bottom of the joists, thus affording the necessary support until the concrete has sufficiently set up. Steady progress is made through the job and the best possible results may be obtained in the organization of labor. A desirable feature in connection with pouring concrete during cold weather, is that the heat from salamanders may be transmitted through the Steelforms directly to the concrete.

Economy: As previously explained, the design of reinforced concrete joist floor used with Meyer Steelform Construction requires a minimum of concrete and steel. Concrete and steel are used only where they are effective in resisting stresses,—all non-carrying concrete is eliminated. The spacing of joists being a maximum, the full benefit of the tee section is obtained with a minimum of material. It is used with equal economy in connection with the concrete frame or steel frame building, and effects enormous savings in dead load with consequent savings in the carrying members of the building in comparison with the clay tile concrete joist floor and other types of construction.



Meyer Steelforms in Place



Open Concrete Joist Ceiling

CECO FIREPROOFING MATERIALS

CONCRETE ENGINEERING COMPANY



The great rigidity of Meyer Steelforms permits the use of a simple and inexpensive system of open wood centering, details of which are shown on page 8. Lines of centering are provided only beneath the joists, the intervening space being left open, thus effecting a very decided saving in the cost of the form work over other types of floor construction.

With the minimum of materials required with Meyer Steelform Construction, the labor costs are obviously reduced.

And as economy in form work is in proportion to the re-use obtained, the economy effected through the removal and re-use of Meyer Steelforms is easily appreciated. The Steelforms are offered to the builder on a rental basis, thereby eliminating his investment expense.

SERVICE

Engineering Department: The function of this Department is to prepare the most economical design for any type of reinforced concrete structure. The most economical construction is always used, the only exception being when the owner or builder prefers some other design. Any type of design can be prepared, and photographs showing buildings in which the Engineering Department have used flat slab and beam and girder designs, are shown on page 19. Complete details, drawings and specifications of concrete construction are furnished, the drawings showing clearly the exact location of the reinforcement and detailed sizes of all concrete work. Recommendations, estimates, preliminary layouts, etc., of all types of concrete construction, are a part of the service and incur no obligation.

CONTRACT DEPARTMENT: This Department handles the labor of erection or installation, on the job, placing and removing Meyer Steelforms (on open wood centering erected in place by others), furnishing, fabricating and placing reinforcing steel and column spirals. The field organization consists of foremen who are thoroughly familiar with this class of work, and entirely competent. The work is executed under their supervision and exactly in accordance with the drawings prepared by our Engineering Department and approved by the architect or engineer in charge.

The Lathing Division of the Contract Department, furnishes and erects metal lath furring of every description, ceilings, partitions, corner beads, base beads, metal picture moulding, etc. Special attention is paid to difficult ornamental furring.

Complete stocks of reinforcing and fireproofing materials are carried in our several warehouses. Our fabricating facilities are the best, and immediate shipments can be made from stock.



Placing Meyer Steelforms



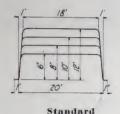
Erecting Lath Ceiling



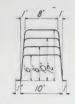


SIZES OF MEYER STEELFORMS

INTERMEDIATES



13'



Special

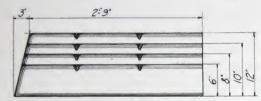
Standard intermediates furnished in 1, 2 and 3 foot lengths.

Special intermediates furnished only in $\boldsymbol{3}$ foot lengths.

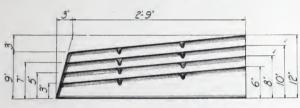
Straight endforms furnished only in 1 foot lengths, 10, 15 and 20 inches wide.

Tapered endforms furnished only in 3 foot lengths, 20 inches wide at open end.

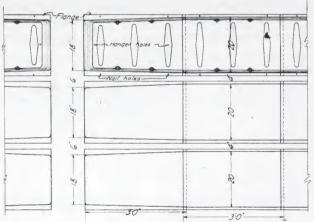
TAPERED ENDFORMS



Single Tapered Endforms



Double Tapered Endforms



Plan Showing Application of Tapered Endforms

SPECIFICATIONS

FLOOR CONSTRUCTION (See Details on Page Eight)

The floor construction in general to be Meyer Steelform Construction, in accordance with the design and practice of the Concrete Engineering Company, Omaha, Nebraska. This construction involves the use of removable Steelforms in the floor slabs, forming a slab and joist construction, the Steelforms to be placed upon open wood centering. Steelforms shall remain in place for a period of seven days after the pouring of concrete, and shall be removed only upon notification of the architect or engineer. Severe weather conditions may necessitate leaving the Steelforms in place for a longer period of

time. Temporary braces, or supports, shall be erected after the removal of Steelforms to properly support the floor construction until the concrete has thoroughly set.

The Steelforms shall be manufactured of No. 16 gauge sheet steel and shall have depressed ribs in the top surface to effect the necessary rigidity. They shall be provided with nail-holes along the lower flanges to permit nailing to the open wood centering, and shall have 3-16" round openings in the center of the top surface of each Steelform to receive wire hangers for the lath ceilings when attached directly to the concrete joists.

CEILING CONSTRUCTION (See Details on Page Eight)

Where lath is to be applied directly against the bottom of concrete joists, or suspended to a distance not exceeding 6" below the joists, place No. 10 gauge soft galvanized wire hangers through the top surface of each Steelform at 3'0" c-c, providing a loop in each hanger to engage the concrete. \(\frac{4}{4}\)" CECO cold rolled channels shall be then erected, running parallel and between the rows of joists, cross furred with \(\frac{4}{4}\)" round steel pencil rods at 13\(\frac{1}{2}\)" c-c, running transversely to the joists and carrying channels. Lath shall then be applied, using CECO Quality or Economy——gauge expanded metal lath, painted (or galvanized), or CECO——gauge wire lath, painted. All

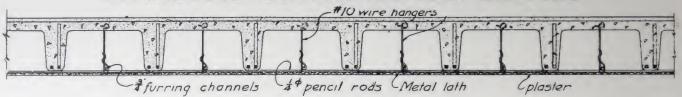
tying to be done with 18 gauge soft galvanized wire.

Where lath ceiling is to be suspended a greater distance than 6" from the concrete joists, place ¼" round mild steel hangers at 4'0" c-c in the concrete joists through holes bored in the wood centering, with a loop in the hanger to engage the concrete. 1½" CECO cold rolled carrying channels shall then be erected at 4'0" c-c, parallel to the joists and cross furred with ¾" CECO cold rolled channels, running at 13½" c-c transversely to the carrying channels, all tying to be done with 14 gauge soft galvanized wire. Lath shall then be applied as with attached ceiling, using 18 gauge soft galvanized wire.

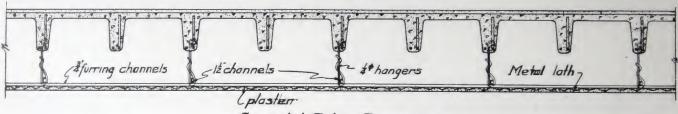




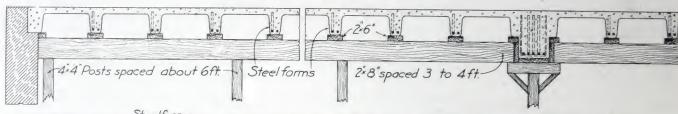
STANDARD CONSTRUCTION DETAILS

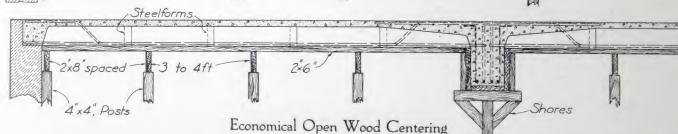


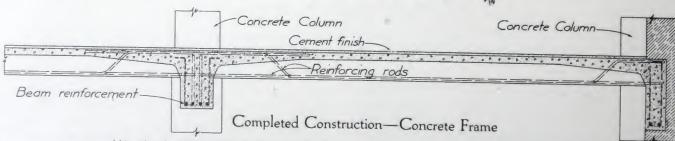
Attached Ceiling Construction

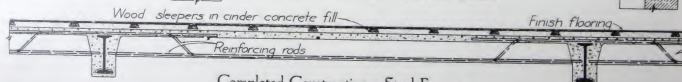


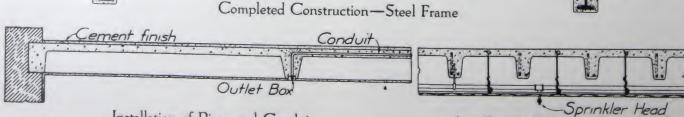
Suspended Ceiling Construction











Installation of Pipes and Conduits

Installation of Sprinkler System
(Showing concealed pipes)





CONCRETE SPECIFICATIONS

Prosecution of the work under this head by the Contractor is to be governed entirely by the specifications as hereinafter stated, and to follow measurements and details, together with all notes that may appear on the drawings accompanying these specifications. Materials are to be used in the manner specified, and are to be the best of their respective kinds. The concrete and steel sizes shown on the plans must not be altered under any circumstances without the written consent of the Architect.

CEMENT

Cement to be used in this construction shall be a standard brand of Portland Cement, approved by the Architect, and shall conform in every respect to the requirements and specifications of the American Society for Testing Materials.

All cement shall be tested by a competent Inspection Bureau, approved by the Architect, and all expense of testing shall be borne by the Contractor. Reports of the testing by the Inspection Bureau shall be furnished to the Architect before the cement is used. No cement which fails in any of the above requirements shall be used in this work.

Provision shall be made for the storage of the cement so as to exclude as far as possible all moisture from the material, as no cement that has become lumpy may be used.

SAND

The sand shall be clean, coarse, sharp, thoroughly screened, and free from all foreign substances.

BROKEN STONE OR GRAVEL

The aggregate shall be broken stone or screened gravel. Broken stone shall be hard, thoroughly screened, clean and free from dirt. It shall be crushed so that its largest dimension shall pass through a ring of one inch in diameter. Gravel shall be clean, free from dirt and sand, and shall range in size from that of a pea to one inch.

WATER

The water used in mixing concrete shall be free from oil, acid, alkali or organic matter.

PROPORTIONS

Concrete for slabs, beams, columns, footings, etc., shall be mixed in the proportion of one part cement, two parts of sand and four parts of broken stone or gravel. The sand and broken stone or gravel shall be carefully selected, and the proportions so regulated as to obtain a mixture of maximum density, thus reducing the voids in the aggregate to a minimum.

MIXING AND PLACING

The mixing of concrete shall be thorough and complete, as the maximum density and greatest strength depend largely upon thorough and complete mixing. The mixing shall be done in a Batch Machine Mixer, and shall continue a minimum time of one and one half minutes after all the ingredi-





ents are assembled in the mixer. The materials shall be mixed wet enough to insure the concrete to flow sluggishly into the form and about the steel reinforcement. The quantity of water is of the greatest importance in securing satisfactory concrete. Too much water is as objectionable as too little.

Concrete, after the completion of the mixing, shall be conveyed rapidly to the place of final deposit. Under no circumstances shall concrete be used that has partly set. The concrete shall be carefully spaded until all the ingredients are in their proper place, so as to insure a minimum amount of voids. The concrete floor shall be wetted down or sprinkled for several days after pouring, at intervals to be specified by the Architect.

During extremely cold weather the concrete is to be carefully protected to prevent injury from freezing. The water used in mixing concrete shall first be heated either by a steam pipe coil in the water, or by injecting steam directly into the water. The temperature of the water should be about 100 degrees Fahrenheit when used in the mixer. All sand and stone should be heated as uniformly as possible, either by means of direct heating from a fire, or by a steam jet. It is important that no frosty material be used in this work. Each batch of concrete coming from the mixer should be placed in the work immediately after mixing, and the temperature of concrete in place should not be below 50 degrees Fahrenheit. The work should be protected by housing the entire area with canvas. Salamanders shall be used below the floor, same being put into operation several hours before the placing of concrete to insure an even temperature. Sufficient salamanders should be used to maintain a temperature of about 60 degrees Fahrenheit for a period of forty-eight hours.

When concreting is once commenced, it must be carried on vigorously to completion if possible, but if concreting must be stopped before the entire floor is completed, the stop shall be made in the center of beams and center of floor slabs. The plane where concrete work is stopped must be vertical and at right angles to the direction of the beams or slab. In no event shall work be terminated in beams or slabs where future shearing action becomes great, nor at their ends or directly under a heavily concentrated load. Before any concrete is placed against concrete already set the latter shall be carefully cleaned and thoroughly wetted, after which the surface shall be treated with a cement wash.

Should any voids occur in the concrete after the forms are removed, same are to be neatly repointed with cement mortar in the proportions of one part cement to two parts fine sand.

FORMS

All forms for this work shall be substantial and unyielding, properly braced and supported so that the concrete may conform to the design, and be sufficiently tight to prevent the leakage of cement and grout. All forms shall be left in place until the concrete has attained sufficient strength to support itself and any super-imposed loads with safety. The work shall be carefully inspected before the forms are removed.

REINFORCING STEEL

All reinforcing steel shall be hard grade steel, rolled from new billets and shall meet the Manufacturer's Standard Specifications. Reinforcing steel is to be accurately and carefully placed under the supervision of an expert engineer, who shall at all times be in charge of the placing of the reinforcing steel. The steel in slabs and joists shall have not less than one half inch of concrete covering. Ceco Bar Chairs shall be used in all concrete joists, slab and beam constructions to hold the reinforcing bars in their proper position before the pouring of concrete. In beams, columns and footings, reinforcing steel shall have at least one and one-half inches of concrete covering.

For specifications of Meyer Steelform Construction and Ceiling Construction, see page 7.





EXPLANATION OF TABLES

The accompanying tables of Meyer Steelform Construction cover all the spans and loads in ordinary use. Tables are given for all depths of Steelforms, namely: 6", 8", 10" and 12". 2", 2½" and 3" thickness of concrete slab over joists are given in combination with 4", 5" and 6" joists. Combinations of depth of Steelforms, thickness of concrete slab, and width of joists have been selected with a knowledge of the probability of their use.

Figures given in the tables are safe total loads in pounds per square foot. The tabulated loads include the weight of the concrete joist and slab construction, which is given in each table as "Weight of slab and joist per square foot." In arriving at a safe Live Load, allowance should be made for the dead weight of the concrete construction, the finish on the floors, the ceilings, and the partitions.

The Following Symbols Are Used

fc-designates maximum extreme fibre stress in concrete.

fs-designates maximum tensile stress in steel.

Ø-designates round bars.

—designates square bars.

The tables have been computed for a maximum tensile stress in the steel of 18,000 pounds per square inch, and a maximum extreme fibre stress in the concrete of 700 pounds per square inch. Combinations of bars have been selected for bending moments of WL-8, WL-10, and WL-12 depending on whether the joists are simple spans, continuous at one end, or continuous over both supports.

The safe loads below and to the left of the lower left hand zig zag line produce shearing stresses

in the joists not to exceed 40 pounds per square inch. The safe loads below and to the left of the upper right hand zig zag line produce shearing stresses not to exceed 60 pounds per square inch. Loads to the right and above upper zig zag line produce shearing stresses in excess of 60 pounds per square inch. Shearing stresses have been computed on the basis of the use of our Standard Tapered Endforms. (See page 7.)

Ceco Temperature Fabric or ½" round bars should be used for shrinkage or temperature reinforcement in the concrete slab at right angles to the joists.

BAR SIZES AND WEIGHTS

Size	Round	l Bars	Square	Bars
13126	Area	Weight	Area	Weight
1/4"	.049	.167	.063	.212
3/8"	.110	.376	.141	.478
1/2"	.196	.668	.250	.850
5/8"	.307	1.043	.391	1.328
3/4"	.442	1.502	.563 ·	1.913
7/8"	.601	2 044	.766	2.603
1"	.785	2.670	1.000	3.400
11/8"	.994	3.380	1.265	4.303
11/4"	1.227	4.172	1.563	5.313



CECO TIREPROOFING MATERIALS CONCRETE ENGINEERING COMPANY MEYER STEELFORM CONSTRUCTION



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th of	22 23 24			106	140	170 156 143	201 184 169	238 218 200	273 250 230			112	135	164	194	229 2/0 /92	262 240 220
Length	25 26 27				99	/32 /22 //3	156 144 133	184 170 158	211 195 181				96	127	150 138 128	177 164 152	203 188 174
	28 29 30					98	124 116 108	/47 /37 /28	168 157 147					94	120	/4/ /32 /23	162 151 141
DEF	PTH						-		RM	+2		DNC.			/		
Joi	575	V	Vt. of.	slab d	Jois and J	oist p	24"C	ft = 5.	4/bs.			"Joi				ft.=5.	9/6s.
Bors	2/8	10/4/20	2.80	WIA WIA	2-14	2-70	1-10			2000	WIA W/	2-11	2-70	1-100			
of	刻	02-2	2-5	2-50	210014	WIANIE	MANIE	5-10		2,00	2.00	NIO WA	WIA VIE	WAY 10	5-14		
Size	12/2/	1-81W	2-70	1-120	2.50	7 7 7 NIOWIA	2-30	WIAT ISO	2-7 0	2-20	12/2/10	2,00	NIONIA	2.30	WIA VIG	2-7	1-10
+	13 14 15	238 205 178	228							294 253 220	322 28/	342	4/8				
in fee	16 17 18 19	157 139 124	200 178 158 142	202	247	27/				194 173 153 137	247 219 195 175	266		342	363		
of span in feet	20 21 22	111	128 116 106	182 164 148 135	222 200 181 165	271 244 222 202	26/			124	158 143 130	192	235	277	328 297	377	392
	23 24 25		97	124	151 139 128	184	218 200 184	218	252			145	177 163 150	210 192 177	248 228 210	284 261 240	327 300 276
Length	26 27 28			97	118 109 102	144 134 124	158 147	202 187 174	216				139	163 152 141	180	206	237
	30				95	108	137		187						156		192



CECO HERPROOFING CONCRETE ENGINEERING COMPANY ON STREET CONSTRUCTION



0-				-	-	Loc				11							
DEF	PTH		1	"JOI.	0 0	111	LIO	KIVI	+62	CO	770	"Jo	ICTS	251	100		
JOI.	575	Wt. c	of slab	and	joist	per 3	gft.	4416	5.	Wt.	of slot	b and	joist	pers	gft.	46 165	
Bors	3/8	1-16	2-15	1 00 0/A	1-0014 410014	1-016				1-20	2-50	PIODWA *	1-000 A	2100-100			
fo	3/0	1 1 0 m	5-70	2-50	2-50	1- NO 1-1	1-1818			P 100-1	5-70	5-8-2	2-150	1-100 WA	סריסוני		۰
Size	W/2	2.30	1-040 N-040	5-70	1-10	2-50	1- Bludais	2-30	1-4 d	2-30	1-019	5-7,0	1 4100	2.50	1001A	2-30	1-100
	10	166	231	296	380	382				160	222	284 235	365	368			
feet	12	115	161	206	265	321	391			111	154	197	254		376		
	13		137	175	226	273	333	240			131	168	216	_	320		
i	14		118	151	195	236	287	340 296	350		113	126	187	197	276	<i>326 284</i>	3.3
rods	16			115	150	180	220	260	308			111	145	174	2//	250	29
	17				133	160	195	230	27 <i>3</i>				114	154	187	197	26
to	18				770	128	156	184	218				114	123	150	177	2/
4	20					115	141	166	197					111	135	160	18
9 t	21						116	<i>151 13</i> 7	178						1/2	145	15
Length	23						.,,	707	,00						,,,_	,,,,	75
7	24																-
	26																
						// 07			011	2/	" ~ ~	100	0.57				
DEF	PTH			- 44					ZM.	+ 22		NC.					
101.	575	L	Vt of	slab d	JOIS and j	oist p	24" (er sq.	ft= 4	91bs.		Wt. of	" JOI. slab d	STS and j	25"	CC per sq	ft.=5	216
Bors	7/10	2.5	NOWA	1 PANONYA	1000	2-2	1-1			2-15 4	NOWIA	PIONA	יומיומי	2.7.0	1-10		
of	20	2-2.	2-15	2.50	- Alwaky	WIA Y BO	WAY W	91-2		5-70	2-5	2.50	NIB WIA	WIANG	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5-14	
Size	777	1-130	2-70	10/20	2-50	- would	2.130	WIA 1-10	2-20	1-1- W-00'W	2-7-	owske.	2-50	NOW!	2-30	with to	0 70
		292	375							281							
\ 1	12	241		332							298	318		-	-	-	-
feet	13	173		284	347					166		27/					+
	14		191	244	299	364	271			143	184	234	288	350			
11	15	130	146	187	260	<i>316 278</i>	374	391		125	141	179			360		-
spon in	17		130	166		246			396		125				280		
3	18			148	181	220	260	309	356			141	174	212	250	297	73
of	19			/33	146	197	233		320		-	127	156			266	
2	2/				133	161	191	227	260				128	155	183	215	2
4	22			-		147	174	207	238	1	-				167		
464	7.3					1,33	1/24	IMA	1111					1/2/	1 / 5 2	197	
Length	23					135	146	184	200					/30	140	167	11.
Lengt						733	146		200					/30	140		1/



CECO INTERPROPRIED CONCRETE ENGINEERING COMPANY

MEYER STEELFORM CONSTRUCTION

Table of Safe Total Loads in Pounds per Saft fe= 700 fs=18000

_																									
DE	PTH	1	411	/-					LF																
Joi	STS								-54		of.	slob	2,0	TS 1st p	Loers	5 °C 9 ft=	580	W	of s	01.	573 E JOI	5 L	16	CC ft=6	2#
Bors	3/0	318-2	1 10 M	41004	- ON	2-7 0	1-100/-/			4014	1- Ba		2-1-2	1.100				ANONA B &	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1-100 P	2.80	101			
of	10/	. 2-2.	2-15	2-55	- Oliv	4-1	WAY!	2-10		3:50	2-50	* 10 MA	- B - IG	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	41-2			2-54	200	00-	0 % mid > 1K	441	\$1-2		
Size	12/2		5-20	IN MI	2-50	ار مالام	2-30	44-1	2-50			2-50			ł	2-78.	1-10	2-50	100		4004		1 1 14 M		
	13			345 297	3/1						332	347								387	472				
	15	157										302									406 353				-
	16	138	177	228	276	338				170	219	266	325					163	210	254	311	365			
feet	17		157	200 180	218	300	352					235 210						120	184	225	276 246	324			-
	19		740		196					155		188						129			221				-
ć	20				179	216	254	302	349		143	170								163	199	234	278	321	
00	27	-	-	-					317							305					181				
ofspan	23	1	-	-	140				264			140	157	185	220	277 254	291	-		134	165			243	
of	24					150	177	210	242							232								223	
4	25					138			223				133			214					127			206	
6	26	-					150	-	191		_	_		145		198 183						/38		190	
0						1											196		-		-			176	
Length	28							154	178						140	///	136						146	164	18
Len	28 29							/54	166						140	160	183						142	153	17
7	28 29 30						121		166 155	5/	-	20	1	, 2		160 149	183 171		67	-			142		17
7	28 29	-	,,			-		15	166 155						1"(160 149	183 171 M			-				/53 /43	17
DE	28 29 30	4	"Of s.	101.s	57S	5 2	4"	5	166 155	5	7"	101	57.	5	½" (25	160 149 O.	183 171 M	6	"	01	575	5 A	26'	153 143	17
DE. Joi	28 29 30	4	of s	6b 0	nd.	5 2	4"	5	166 155 TE	W	ofsk	1013	57.	5	½" (25	160 149 O.	183 171 M	6 Wt	"Of sk	01.	1015	t per	26'	153 143	10
OF Bors OC	28 29 30 7PTH /STS	A WH	of s	lab o	2-7.	10151	per	5	166 155 TE	WH	of sh	1018	57.	5 per	½" (25	160 149 O.	183 171 M	WHO N	of sla	01.068	1015	t per	26'	153 143	10
DE. Jo.	28 29 30 7PTH 1STS	AW+ 1081-7 012	of s	1-1000	1-10 2-10 pu	5 2 101511 101511 101511 101511 101511 101511	1-2 per	CC	166 155 TE	N. 2-60 /- 10 00 00 00 00 00 00 00 00 00 00 00 00	of shapping	101.00	S7.	S per -2 -2	25	160 149 10. CG+=6.	183 171 NC	6 W 6 0 - 7 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5	of sla	01.068.	1015:	t per	26'sqff	153 143 (CC) 4=65	10
OF Bors OC	28 29 30 77H 1STS	ANT 188-7 05-2	0 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2-50 /- Par /- P		5 2 101511 101511 101511 101511 101511 101511	24" per	CC	166 155 TE	1-1-1-0 0 1-1-1-0 0 H	of 5/6	101.00	57.	S per -2 -2	25	160 149 0. C(f) = 6.	183 171 NC	0 10 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	of ske	01.2 sep-2 s	1015:	t per -1-2 91-2	26' safri	/53 /43 /CC 4=65	17
OF Bors OC	28 29 30 57TH 15TS	4 W4 8 8 8 7 7 8 8 8 7 7 7 3 15 272	of 5.	100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		5 2 101511 101511 101511 101511 101511 101511	1-2 per	CC	166 155 TE	WH 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 of sh	101.2	5.70	S per -2 -2	25	160 149 10. CG+=6.	1833 1711 N(6 WH.	10 of sla	1012 1018 1019 1019 1019 1019 1019 1019 1019	1015	per 2-1 91-2	26'sqff	153 143 (CC) 4=65	10
OF Bors OC	28 29 30 30 57S 7X 13 14 15	AW+ 5100 -7 5100 -7 7 5100 -7 7 5100 -7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	of 5.	497 423 370	alavia /	5 2 101511 101511 101511 101511 101511 101511	1-2 per	CC	166 155 TE	WH 140 MIN 1 140 MIN	0 of 5/4	1010 100 & 100 100 & 100 100 100 100 100 100 100 100 100 100	57.015.00 S. 10.00 S.	S + pei	25	160 149 10. CG+=6.	183 171 MC 15#	60 WHO NO	10 of sla	01.06 &	1015	1-20 2-10 T-10 T-10	26'sqff	153 143 (CC) 4=65	16
OF Bors OC	28 29 30 77H 1STS 13 14 15 16	AW+ 8 8 8 7 7 8 15 272 237 210	of 5.	492 423 370 323	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5 2 101511 101511 101511 101511 101511 101511	1-2 per	CC	166 155 TE	WH 140 ME 170 - 2 170 - 2	0 f 5/h	1010 100 2 100 100 100 100 100 100 100 100 100 10	57. poiss	S pei	25	160 149 10. CG+=6.	183 171 MC 15**	60 WHO NO	10 of sk	01.06 & NO	490	1-2 5-10 T-10	26'sqft	153 143 (CC) 4=65	16
Size of Bors OF	28 29 30 77H 1STS 13 14 15 16	AWA 15 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of 5.	10 00 00 00 00 00 00 00 00 00 00 00 00 0	3522	5 2101311 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1-2 -1-2	CC	166 155 TE	W/4 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -	0 f 5/h 0 o f 5/	1010 100 & 100 100 100 100 100 100 100 100 100 100	57. jois: 14	S per 2 %	25	160 149 10. CG+=6.	183 171 MC 15**	60 WH.	10 of sh	015 & 100 &	1015 1015 1015 1015 1015 1015 1015 1015	450	26' sqfi	153 143 (CC) 4=65	13
feet Size of Bors O A	28 29 30 77H 1STS 13 14 15 16	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	10 10 10 10 10 10 10 10 10 10 10 10 10 1	3522 314 282	5 L (01311	per 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	CC	166 155 TE	3390 3336 293 228 204 183	100 of Shi	1010 106 & 100 100 100 100 100 100 100 100 100 100	57. 1015: 1015	5 per - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -	25	160 149 10. CG+=6.	183 171 MC 15**	60 WH. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	" of skill was 12 12 12 12 12 12 12 12 12 12 12 12 12	10/2 10/2 10/2 10/2 10/2 10/2 10/2 10/2	1905 1915 1915 1915 1915 1915 1915 1915	4500 36Z	39ft	/53 /43 /CCC ==65	77
Size of Bors OF	28 29 30	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3522 314 282 254	370 3370 3332 300	per 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	CC	166 155 TE	390 336 293 258 204 183 165	100 of Shi work 12 12 12 12 12 12 12 12 12 12 12 12 12	100 100 100 100 100 100 100 100 100 100	57. 1015 1	S per 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	25	160 149 10. CG+=6.	183 171 MC 15**	60 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	" of skill wo 18 18 18 18 18 18 18 18 18 18 18 18 18	10/2 10/2	490 430 380 376	450 450 450 362 326	26' 3qff	153 143 143 143 143	1)
in feet Size of Bars O 30	28 29 30 77H 1STS 13 14 15 16 17 18 19 20 21	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3522 314 282 254 231	370 332 332 372	per 2	CC	166 155 TE	3900 3336 293 228 204 183 165	1900 of 5/4 1900 of 5/4 1900 of 1900 o	5000 435 382 271 244 222	515 450 358 320 289	S per 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	25	160 149 10. CG+=6.	183 171 MC 15**	6 WH 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6	190 of ships	10/06 & 100 A 100	4900 4303 3803 276 250	4500 4500 362 3962	4/6/37/3342	153 143 143 143 143 143 143 143 143 143 14	77
span in feet Size of Bars O A	28 29 30	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	100 c c c c c c c c c c c c c c c c c c	352 352 314 282 254 200	3700 3322 3300 272 248	293	Sqff	166 155 TE	3900 3365 293 228 204 183 165 150	00 of 5/6	100 100 100 100 100 100 100 100 100 100	57. 515. 515. 450. 450. 289. 262. 239.	5 per -2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	25	160 149 10. CG+=6.	183 171 MC 15**	6 WH 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6	900 18 18 18 18 18 18 18 18 18 18 18 18 18	100 8, 10	4900 4300 3800 2766 2500 2288	450 450 450 362 376 276	416 375 342 310	153 143 143 143 143 1393 3356	77
span in feet Size of Bars O A	28 29 30	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	492 425 370 287 287 208 188 172 157 144	3522 314 282 2254 276 192 176	370 332 332 332 332 272 248 227	293 268 246	Sqff 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1666°	3900 3336 2238 204 183 165 150 136	100 of 5/4 100 of 5/4 100 of 1 100 of 1 10	5000 43.5 382 271 224 222 202 183	515 515 450 450 450 289 262 239 262 239 200	3400 3340 282 258 236	25	160 149 10.	183 171 171 15#	6 WH 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6	392 3340 299 217 173 158 145	100/2 100 8 100 8 100 100 100 100	490 430 3380 276 250 228 703	450 450 362 362 296 270 247 278	26' 3qff	153 143 143 143 143 133 133 133 133 133 13	1)/6
of span in feet Size of Bors O A	28 29 30	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	492 423 370 287 2287 2286 172 157	352 352 314 282 254 276 192 176 363	370 332 300 227 208 192	293 200 246 227	284 250	1666° 155	3900 3336 2238 204 183 165 150 136	100 of 5/4 100 of 5/4 100 of 1 100 of 1 10	5000 435 382 340 271 244 222 202 170 156	515 515 450 450 450 450 450 450 450 45	3400 3400 282 258 218	252	760 149 70 70 71 71 71 71 71 71 71 71 71 71	/83 /71 // 5#	6 WH 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6	392 3340 299 217 173 158 145	4800 418 367 325 234 193 177 162	4900 4300 3800 2760 2280 7092 7077	450 450 40 33 22 27 27 27 27 27 27	4/6/37/5 34/2 3/2 2/2 2/2 2/2 2/2 2/2 2/2 2/2 2/2 2	153 143 143 143 143 143 143 143 14	
of span in feet Size of Bors O A	28 29 30	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	492 425 370 287 287 208 188 172 157 144	3522 314 282 2254 192 176 350	370 3370 3372 248 227 208 277	293 293 266 247 210	284 250 240	1666° 155	3900 3336 2238 204 183 165 150 136	100 of 5/4 100 of 5/4 100 of 1 100 of 1 10	5000 435 382 271 222 202 185 170 156 145	515 450 400 358 320 289 262 239 262 200 171	3400 3100 282 236 202 202	25 25 274 274 274 274 274 274 274 274 274 274	789 268 268	783 171 171 5 # 303	6 WH 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6	392 3340 299 217 173 158 145	100/s. 200 200 200 200 200 200 200 200 200 20	4900 4300 3300 2700 707 707 707 707 707 707 707 707	450 450 40 362 296 270 247 278 209 193	4/6 37/5 342 3/0 22/2 22/2	153 143 143 143 143 1393	7
span in feet Size of Bars O A	28 29 30	4 WH 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	of s.	492 425 370 287 287 208 188 172 157 144	3522 314 282 2254 192 176 350	370 3370 3332 300 272 248 227 208 192 175 153	293 268 227 210 181	284 250 222 200 222 200	1666° 155	W/4 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	100 of 5/4 100 of 5/4 100 of 1 100 of 1 10	500 43.5 382 340 271 224 222 202 185 170 156 145 134	515 515 450 450 289 262 239 262 239 268 200 185 171 158 158 171 158 158 158 171 158 158 158 158 158 158 158 15	3400 3400 282 258 208 2018 2018 2018 2018 2018	274 252 234 252 234 202 202	7899 2898 248 237	783 171 171 5 # 303	6 WH 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6 N 6	392 3340 299 217 173 158 145	100/s. 200 200 200 200 200 200 200 200 200 20	490 430 330 276 250 228 709 192 177 163 151 141	450 450 450 203 362 270 247 278 209 (193 179	266 3qfn 3416 375 342 230 221 204 204 204	153 143 143 143 143 1393	722



CECO TIREPROOFING CONCRETE ENGINEERING COMPANY ON STREET CONSTRUCTION



	Table	01	301			======								5-700			
DE	PTH		E "			EEL,		M+	3"(01				211/	-		
101.	575	W.	of slob	JOIS and	joist	25" (igft =	52 16	5.	Wt.	of slot	JOIS and	joist	26"C	igft=	541bs	
Bors	200	2-15	- B	1- 1- 1- 1- 1- 1- 1- 1- 1- 1- 1- 1- 1- 1	p b	9	1-10			2-154		1000 H	0 6	2-70	1-104		
Size of	3/0	2-10	2-15	2-50	1-80 N	WA VID	1-14 P	5/4		2-2	2-15	2-50	PIED -		WA.10	5-14	
5	12/ 12	1.00 ls	2-70	0 000	2-150	NIOD WIA	2-30		2-7.	1-00-1	2-20	11/0	2.50	010014	2-30	1-410 00-40	2-8-
*	10	237 196 164	250	389 321 270	392	403				227 187 157		<i>378</i> <i>312</i>	376 3/6	387			
in feet	13 14 15	140	-		280 242	344 296	349 304	360		134	172	773 193 168		330 284	334	344	
span in	16 17 18			152	185	227	267	316 280 250	363 322 287		-	147	177	218 193 172	256 226	303 268	300
of	19 20 21				140	161	189	224	257 232 211				740		181	240 215 194 176	275
Length	22 23 24 25						700	167	192						743		184
	26																
DE	PTH				8	"57	EEL	FOR	2M.	+3"	CO	NC	QE7	E			
Joi	ISTS	ı	Nt of	5" slab	JOIS and J	orst p	25" (er sq	C ft = 50	8/65		6' Wt. of.	1 JOI. 5/ab d	575 and J	26"	CC per sq	ft:=61	165.
Bors	20	NIOWA	NOWIA	2000	2-30	1-10				100014	SIO UNA	100 mg	2-70	1-10			
Size of Bars	77/0/	200	2-50	100 mlA	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1-12 P	5-10			2.50	2.00	1-150	100 mg	who -	2-14		
5	12 / 12 / 12 / 12 / 12 / 12 / 12 / 12 /	5-70	1-12/10	2-50	2100 UA	2-30	14 NO	2-30	1-10	2-70	1-1210	2-50	NIOWIA .	2-30		2-7	- A - /
*	10 11 12 13	379 3/3 263 274	408 343	410						252	390 328 279	395					
in feet	14 15 16	193 168 148	252 219	302 263 231	<i>322 283</i>	334	394				241	290 253	355 309	364	378		
of spon	17 18 19 20		171	205 183 164	251 224	296 264 237	<i>350 312</i>	402 359 321	370			197	240 214 192	284 253 227	334 298 268	386 344 308	357
ength o	21 22 23				164	194	229 208 191	264 240 220	276 253				158	205 186 169 155	220 200 183		292 264 243
97	24 25 26 27						175	702	232						154	193	200





MEYER STEELFORM CONSTRUCTION Table of Safe Total Loads in Pounds per Sq.ft fo=700 fs=18000 DEPTH 10"STEELFORM+3"CONCRETE 5" JOISTS 25"CC 6"JOISTS 26"CC 101575 Wt. of slab and joist per sqft = 64 lbs Wt. of slab and joist persaft= 68/bs المارهالم 1-190 Bors 0014 1-10 0/2 100 NO 100 PIOUA 11 3 11 11 of 20014 000 Solo 0 8 10/2 4100 1-14 1-14 WI4MED 11 2 11 11 3 Plowid 0 00 010 w14 -INPAD | 32 403 | 289 348 422 | 252 303 368 | 221 266 324 383 | 196 236 287 339 402 | 175 210 256 302 358 | 142 170 207 245 290 332 385 | 142 170 207 245 290 332 385 | 147 1 202 240 274 378 367 | 157 185 220 257 297 330 | 157 186 212 246 280 | 177 207 207 230 267 303 | 157 186 212 246 280 | 172 196 228 258 | 159 182 217 240 | 169 196 223 | 158 183 20 | 171 19 614 V100 100 100 | 13 | 345 | 420 | 14 | 300 | 363 | 441 | 15 | 260 | 316 | 384 | 300 | 354 | 420 | 18 | 180 | 220 | 267 | 316 | 374 | 19 | 162 | 197 | 240 | 283 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 384 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 335 | 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000 5-10 180 1-10 100 8/2 3 11 11 2 11 3 1-41W nioquia wiar ioo OF W142100 1-10 1-410 10 5-14 1-10 5-10 1-10 70 1 11 11 11 NOW4 1-190 019 w14-100 w14 N100 12/2 1-10 5100 100 BIG 100 WALK NOWA WIA 3 11 11 3 11 14 422 405 494 368 450 324 395 287 350 414 256 312 369 436 353 431 15 311 379 446 275 336 396 245 299 354 418 feel 256 3/2 369 436 229 280 332 392 448 207 253 299 354 404 /88 230 272 321 366 425 //// 209 247 292 332 387 440 //// 209 247 292 332 387 440 //// 208 246 280 326 369 4/7 //// 209 239 278 3/4 355 //// 209 239 278 3/4 355 //// 209 239 278 3/4 355 //// 209 239 27/ 306 //// 208 223 253 286 //// 208 236 267 220 268 318 376 429 199 242 286 339 387 180 220 260 308 352 408 164 200 236 280 320 372 422 183 216 256 293 340 385 19 20 21 22 23 24 25 26 27 28 29 | 168 | 199 | 237 | 269 | 312 | 354 | 400 | 168 | 169 | 200 | 229 | 268 | 301 | 341 | 369 | 200 | 229 | 268 | 301 | 341 | 268 | 269 | 268 | 269 | 268 | 269 | 268 | 269 | 268 | 269 | 268 | 269 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268 | 268

172 200 226 256





FLOOR TESTS

City of Detroit

Dec. 18th-1914.

Concrete Engineering Co., S Few Stott Building,

77-9 Michigan Ave., City.

Regarding the floor test upon the Best

Stott Building.

On December 3rd, we tested the third panel from the sest side and second panel from the south side of the third floor. This panel has a span of 18 foot, 6 inches, is designed for 100 lbs. per square foot and 20 lbs. of finish and was 44 days old at the time of test. It was loade by piling bricks in piers 24 x 24 inches, the piers being six (6) inches apart and of sufficient height to give a uniform live load of 220 lbs. per square foot of floor. After about twenty-four (24) hours the maximum deflection was 3/16 inch or equal to 1/1200 of the span. He eracks could be observed. We consider this a very satisfactory showing.

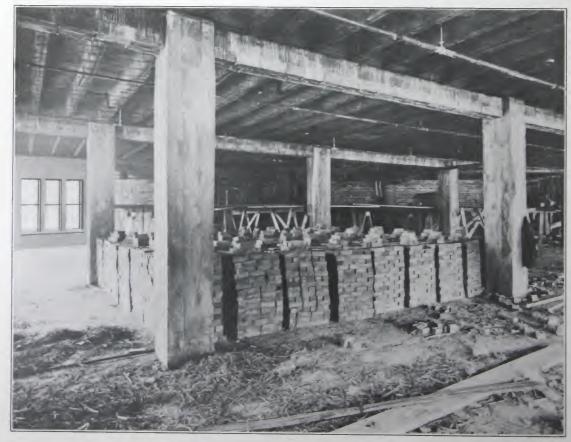
Respectfully yours.

DEPARTMENT OF BUILDINGS.

FB.LDM.

Par Frank Buston CONCRETE ENGINEER.

The David Stott Building, Detroit, Michigan, is a steel frame with Meyer Steelform Floor Construction. A load test was made on a typical panel by the City Building Department of Detroit, amounting to two times the live load for which the floor was designed. The load consisted of independent brick piers which prevented arching to the supporting girders. The deflection was measured and found to be a maximum of 1-1200 of the span. The actual deflection of the floor was probably less than this, owing to the deflection of the supporting girders. This building was originally designed for tile concrete joist floors. The use of Meyer Steelform Construction effected a saving of thirty pounds per square foot, permitting a large reduction in the supporting steel girders and columns. The complete change effected a great economy over the tile design.

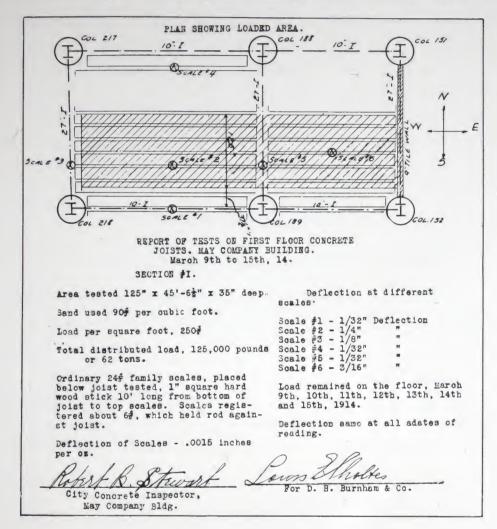


David Stott Building Test, Detroit, Michigan





Report of May Company Building Test-Cleveland, Ohio



Other Standard Designs Prepared by our Engineering Department



Flat Slab Floor Construction Kirschbraun Creamery, Omaha, Nebraska



Beam and Girder Floor Construction Hoagland Building, Omaha, Nebraska





CECO FIREPROOFING MATERIALS

This Handbook heralds an important development in the use of Ceco Fireproofing Materials. In the past our Engineering Department, in conjunction with the prominent architects and engineers throughout the country have specified Ceco Fireproofing Materials in a great many structures of importance, and the Contract Department have handled the erection. In this way, Ceco Fireproofing Materials have been almost exclusively used in the work handled by this Company alone. With this wide use of Ceco Fireproofing Materials, we have had every opportunity to witness the perfect satisfaction caused by the economy and quality of the materials, both as regards their initial cost, and the cost of the subsequent installation.

"Nothing succeeds like Success,' for all concerned, and we have been aiming to make our service and distribution methods so comprehensive that Ceco Fireproofing Materials might be used, not only by our Contract Department, but by any one, in any building, large or small.

The builder is therefore, now enabled to secure Ceco Fireproofing Materials from his dealer, or if the dealer cannot furnish them, immediate shipment of his requirements can be made from one of our warehouses.

Our service is distinctive in that it is entirely complete. Through your dealer you may secure Ceco Fireproofing Materials. And we do more than supply the materials. When desired through the Contract Department, Ceco Fireproofing Materials are installed or erected on the job, in accordance with the standard methods, and to meet the approval of the architect or engineer.

In the following paragraphs we have presented a condensed description, with illustrations, of the various Ceco Fireproofing Materials, and their uses.

CECO EXPANDED METAL LATH

GENERAL ADVANTAGES

Since three coats of plaster are generally used with Ceco Metal Lath, a solid, uniform coat of plaster is obtained throughout and the wall is an excellent non-conductor of heat, cold and moisture. Due to the fact that Ceco Metal Lath is always furnished painted (galvanized if desired) and is further protected by the plaster, an absolutely permanent base and reinforcement for the plastered interior, or stuccoed exterior, is assured. Unlike wood lath, there is no particular expansion or contraction with Ceco Metal Lath, and the subsequent cracking and falling away of the plaster is entirely eliminated.

Ceco Metal Lath imbeds itself in the plaster and retains its grip indefinitely. It does not absorb moisture, hence does not rust and stain the plaster, or collect dust on the plaster surface. With the Page Twenty

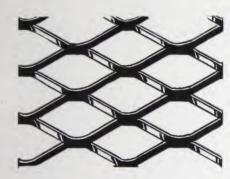




thorough distribution of metal through the plaster, all cracking is eliminated and decorations may be applied immediately without difficulty. Ceco Metal Lath reinforces and the plaster protects,—together they effect a permanent, fireproof construction, for ceilings, partitions, etc.

Being flexible, Ceco Metal Lath is ideal for use in ornamental work of every description. It is also particularly adapted for use in partitions. A permanent, soundproof partition of great strength may be constructed using Ceco Metal Lath and Ceco Cold Rolled Channels, or Ceco Prong Studding, which will effect a considerable saving in floor space and dead load. Cheaper lathing materials may be secured, but considering every factor of expense—maintenance, permanency of investment, etc.—Ceco Metal Lath is undeniably the most economical and satisfactory base and reinforcement for plaster and stucco work of every description.

CECO ECONOMY LATH



This style is an especially economical, light weight lath of small mesh, insuring a thorough key and minimum coat of plaster. It has a twist or slant in the strands of the mesh which prevents shearing of plaster and increases the rigidity of the sheets. Made from highest grade, open hearth steel, and furnished painted, galvanized after expansion, or in copper iron alloy. It has a wide range of weights and gauges, suitable for practically every type of construction.

Gauge .	Size of Sheet Inches	Weight per Sq. Yard	Yards per Sheet	Sheets per Bundle	Yards per Bundle
No. 18	21"x97"	5.50 lbs.	$1\frac{1}{2}$	14	21
No. 20	21"x97"	4.15 lbs.	$1\frac{1}{2}$	14	21
No. 22	21"x97"	3.40 lbs.	$1\frac{1}{2}$	14	21
No. 221/2	21"x97"	3.33 lbs.	$11/_{2}$	14	21
No. 23	21"x97"	3.10 lbs.	$1\frac{1}{2}$	14	21
No. 24	21"x97"	2.75 lbs.	$1\frac{1}{2}$	14	21
No. 25	21"x97"	2.40 lbs.	$1\frac{1}{2}$	14	21
No. 26	21"x97"	2.10 lbs.	$1\frac{1}{2}$	14	21

Add one pound per square yard to above weights for lath galvanized after expansion.





CECO QUALITY LATH

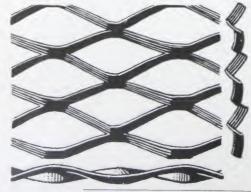


As the name implies, this is a high grade lath to be used where quality is the first essential. It has wide strands of metal and weighs more than ordinary lath. As indicated by the schedule of weights for the various gauges, more steel is used in this lath and a minimum of plaster is obtained, due to the solid rigidity of the lath itself. It does not bend beneath the plasterer's trowel, but stays straight and firm, requiring a uniform coat of plaster throughout. As in the case of Ceco Economy Lath, this material is furnished either painted, galvanized after expansion, or in copper iron alloy.

Gauge	Size of Sheet Inches	Weight per Sq. Yard	Yards per Sheet	Sheets per Bundle	Yards per Bundle
No. 18	21"x97"	8.00 lbs.	$1\frac{1}{2}$	14	21
No. 20	21"x97"	6.00 lbs.	$1\frac{1}{2}$	14	21
No. 22	21"x97"	5.00 lbs.	$1\frac{1}{2}$	14	21
No. 24	21"x97"	4.00 lbs.	$1\frac{1}{2}$	14	21
No. 25	21"x97"	3.50 lbs.	$1\frac{1}{2}$	14	21
No. 26	21"x97"	3.00 lbs.	$1\frac{1}{2}$	14	21

Add one pound per square yard to above weights for lath galvanized after expansion.

CECO SELF-FURRING LATH



As will be noticed from the illustration, Ceco Self-Furring Lath has a corrugation or rib running through its strands which gives it added strength and rigidity, and eliminates the necessity of furring strips. It is especially economical for use in exterior stucco work, and is manufactured almost exclusively for that purpose. It is applied directly to the sheathing boards, or studding, no furring strips being necessary, and a uniform thickness of plaster is a certainty. It is also furnished painted, galvanized after expansion, or in copper iron alloy.

Gauge	Size of Sheet Inches	Weight per Sq. Yard	Yards per Sheet	Sheets per Bundle	Yards per Bundle
No. 24	21"x97"	4.00 lbs.	11/2	14	21
No. 25	21"x97"	3.00 lbs.	11/2	14	21
No. 27	21"x97"	2.80 lbs.	$1\frac{1}{2}$	14	21

Add one pound per square yard to above weights for lath galvanized after expansion.

CECO CRIMPED FURRING



To facilitate the use of Ceco Economy or Quality Lath in exterior stucco work, we are furnishing the Ceco Crimped Furring strips for fastening the lath to the sheathing or studding. It keeps the lath away from the wall allowing room for the plaster to key. Ceco Crimped Furring comes in $\frac{1}{2}$, $\frac{3}{4}$, or 1 widths and lengths desired, from 9 to 14, packed 25 pieces per bundle, made from special analysis steel. Weight, $\frac{1}{2}$ wide, 53.2 lbs. per 1,000 linear feet; $\frac{3}{4}$ wide, 79.8 lbs. per 1,000 linear feet, and 1 wide, 106.4 lbs. per 1,000 linear feet.

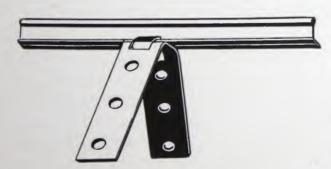
CECO CORNER BEADS



Plastered corners are easily broken and our Ceco Corner Bead here illustrated is designed to afford a steel reinforcement for the plastered corner as well as a guide for the plaster when erected. Ceco Corner

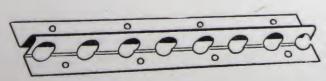
Beads are made of 24 gauge steel, and being galvanized after forming, will not rust and stain the plaster. This is an exceptionally heavy bead. The round openings in the flange permit a strong keying action on the part of the plaster. Clips are furnished if desired, permitting adjustable grounds. Lengths, 6, 7, 8, 9, 10 and 12 feet. Weight, 225 lbs. per 1,000 linear feet. Shipped 10 pieces to the bundle.

CECO RAIL BEADS



Some architects and contractors prefer this type of bead which affords a substantial protection for the plaster corner. It is adjustable for any depth of grounds, one clip per foot of length being furnished. It is especially strong and heavily galvanized. Furnished in lengths 6, 7, 8, 9, 10 and 12 feet. Weight, 130 lbs. per 1,000 linear feet. Shipped 25 pieces to the bundle.

CECO BASE BEADS



This bead is extensively used to separate the cement, or composition base, from the wall plaster above. It affords a straight and sanitary joint of the two materials, preventing their contact and the absorption of moisture. It is made only for ½

grounds, and of 24 gauge steel, heavily galvanized. The following lengths are furnished—6, 7, 8, 8' 6", 9, 9' 6", 10, 11 and 12 feet. It weighs 170 lbs. per one thousand linear feet, and is shipped 12 pieces to the bundle.





CECO COLD-ROLLED CHANNELS



Ceco Cold Rolled Channels, either plain or perforated, are used in conjunction with Ceco Metal Lath in erecting ceilings, partitions, etc., in first class fireproof structures. In comparison with the heavier hot rolled channel, you have much greater tensile strength in our cold rolled channel, weight for weight, than can be secured in the hot rolled channel. They are made from best grade open hearth steel with square shoulders and on account of the great strength and reduced weight, are much more economical than the hot rolled channel. (See details pages 26 and 27.)



PLAIN

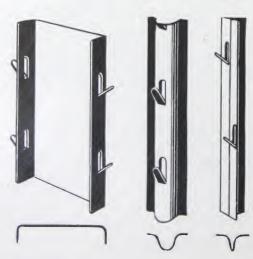
PERFORATED

Gauge	Size	Weight Per M Linear Feet	Size of Flange
16	3/4"	276 lbs.	3/8"
16	7/8"	304 lbs.	3/2"
16	1 "	331.5 lbs.	3/2"
16	11/4"	386.8 lbs.	3/2"
16	11/2"	456 lbs.	3/2"
16	2 "	580 lbs.	3/2"
16	2 "	635.4 lbs.	1/2"
16	11/2"	539 lbs.	$1/\bar{5}''$
16	17/8''	458.2 lbs.	1/5''

Gauge	Size	Weight Per M Linear Feet	Size of Flange
16 18	1½" 1½"	455.8 lbs. 458.2 lbs.	3/8" 1/2"
18	2"	479 lbs.	1/2"
18 18	$2\frac{1}{4}$ " $2\frac{1}{2}$ "	520.6 lbs. 562.3 lbs.	1/2"
18 18	3 " 31/ ₅ "	645.6 lbs. 728.9 lbs.	$\frac{1}{2}$ "

Lengths 9 to 20 feet.

CECO PRONG STUDDING



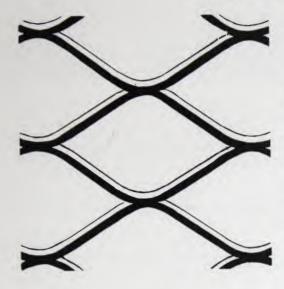
Many builders prefer the prong stud to the plain or perforated channel for use in partitions, both solid and hollow, on account of the minimum of labor required in applying the metal lath. Ceco Prong Studding is designed to meet this demand, made in two gauges, Numbers 18 and 20. As shown in the illustration, the prongs easily engage the mesh of the lath which is quickly applied by simply bending back the prongs with a hammer. Ceco Prong Studding is made with the prongs punched about every $3\frac{1}{2}$ inches. The double studding for hollow partitions comes in 2, 3, 4, 5 and 6 inch widths and 10 and 12 foot lengths, and the small tee studding for solid partitions comes only in $3\frac{1}{4}$ inch widths and 10 foot lengths. (See partition details on page 27.)

NOTE: Lathing accessories, namely galvanized wire ceiling hangers, cut to lengths and bent; 14 gauge, 1" staples; 18 gauge soft galvanized tie wire, etc., can also be furnished promptly from stock.





CECO EXPANDED METAL



Expanded Metal reinforcing is a thoroughly efficient reinforcement for use in the construction of concrete floors, roofs, sidewalks, roads, bridges, sewers, etc. It saves labor, assures absolutely correct placing and practically any desired sectional area of steel can be furnished. The width and thickness of the steel can be varied, so that if the sectional area of steel is specified, the required style of expanded metal can be furnished to answer the purpose. Made in convenient sizes, it is easily handled by one man and large areas quickly covered. No wiring or spacing is necessary. Made in one piece and thoroughly rigid, there is no slipping of joints. The bond with the concrete is absolute. Ceco Expanded Metal is made in the following sizes and sectional areas:

Mesh	Weight per Square Foot	Sectional Area per Square Foot	Mesh	Weight per Square Foot	Sectional Area per Square Foot
3x7 inches	.20 lbs.	.059 inches	3 x6 inches	1.40 lbs.	.413 inches
3x7 inches	.24 lbs.	.072 inches			007.1
3x7 inches	.28 lbs.	.082 inches	21/2x5 inches	.323 lbs.	.095 inches
3x7 inches	.32 lbs.	.094 inches	21/2x5 inches	.430 lbs.	.127 inches
3x7 inches	.36 lbs.	.106 inches	21/2x5 inches	.538 lbs.	.159 inches
3x7 inches	.42 lbs.	.124 inches	21/2x5 inches	.816 lbs.	.241 inches
3x7 inches	.46 lbs.	.136 inches	$2\frac{1}{2}$ x5 inches	.979 lbs.	.289 inches
3x7 inches	.50 lbs.	.147 inches	21/2x5 inches	1.142 lbs.	.337 inches
3x7 inches	.55 lbs.	.162 inches	21/2x5 inches	1.305 lbs.	.385 inches
3x7 inches	.61 lbs.	.179 inches			
3x7 inches	.79 lbs.	.232 inches			
3x7 inches	.85 lbs.	.251 inches	11/2x3 inches	.333 lbs.	.098 inches
3x7 inches	.93 lbs.	.274 inches	11/5x3 inches	.433 lbs.	.128 inches
3x7 inches	1.02 lbs.	.301 inches	11/5x3 inches	.566 lbs.	.167 inches
3x7 inches	1.10 lbs.	.324 inches	, -		
3x7 inches	1.19 lbs.	.351 inches	1 x2 inches	.25 lbs.	.074 inches
3x7 inches	1.28 lbs.	.377 inches	1 x2 inches	.372 lbs.	.109 inches
3x7 inches	1.36 lbs.	.401 inches	1 x2 inches	.500 lbs.	.148 inches
3x7 inches	1.44 lbs.	.425 inches	1 x2 inches	.65 lbs.	.192 inches
3x7 inches	1.53 lbs.	.451 inches	1 x2 inches	.85 lbs.	.250 inches
3x7 inches	1.61 lbs.	.475 inches			
3x7 inches	1.70 lbs.	.502 inches	3/4x2 inches	.379 lbs.	.114 inches

The standard sizes of sheets are 6, 8 and 10 feet long, by 4 and 6 feet wide.

Any special size can be furnished. When giving sizes of sheets name the dimension the long way of the Diamond, first.





CONSTRUCTION DETAILS

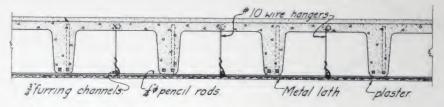
Condensed Specifications

GENERAL: Ceco Metal Lath is to be applied with the long length of the sheet at right angles to all supports. Care must be exercised to apply the lath so that the twist or slant in the strands slopes down and away from the plaster side, thus preventing shearing and dropping away of the plaster. All wiring of Ceco Metal Lath is to be done with 18 gauge galvanized soft wire at intervals of at least 6". Sheets are to be lapped at least ½" at the sides and not less than 1" at the ends.

Attached Ceilings Beneath Meyer Steelform Construction

10 gauge galvanized ceiling hangers inserted in concrete slab before pouring 3'0" c-c; \(\frac{4}\)" Ceco cold rolled channels 2'0" c-c suspended by hangers between joists, cross furred with \(\frac{1}{4}\)" round steel pencil rods wired to

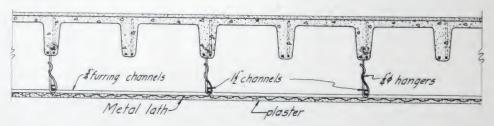
channels at 13½" c-c; —— gauge Ceco Metal Lath, painted, applied using 18 gauge soft galvanized wire. This ceiling may be suspended from joists to a distance not exceeding 6".



Suspended Ceilings Beneath Meyer Steelform Construction

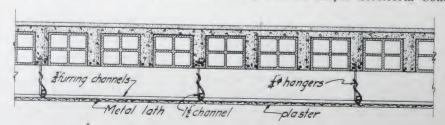
 $\frac{1}{4}$ " round mild steel hangers to be inserted in concrete joists before pouring through holes bored in wood centering at $\frac{4}{0}$ " c-c (both directions); $1\frac{1}{2}$ " Ceco cold rolled carrying channels to be suspended by hangers and cross furred with $\frac{3}{4}$ " Ceco cold rolled

channels 13½" c-c, tying of channels to be done with 14 gauge soft galvanized wire, and —— gauge Ceco Metal Lath, painted, to be applied using 18 gauge soft galvanized wire.



Suspended Ceilings Beneath Clay Tile Concrete Joist Construction

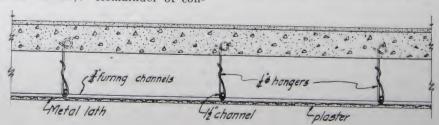
Specifications exactly the same as for the suspended ceiling beneath Meyer Steelform Construction.



Suspended Ceilings Beneath Flat Slab Construction

1/4" round mild steel hangers to be inserted in slab before pouring through holes bored in the wood centering at 4'0" c-c (both directions). Remainder of con-

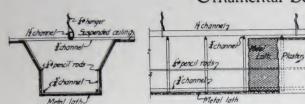
struction exactly the same as suspended ceilings beneath Meyer Steelform Construction.







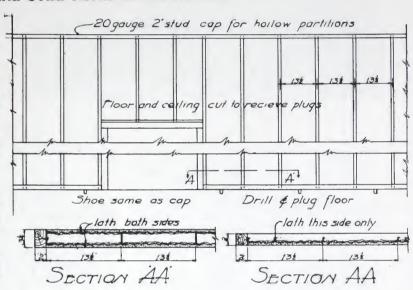
Ornamental Beam (or Cornice) Furring



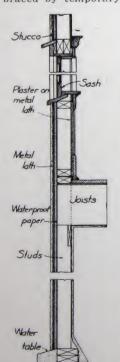
The brackets forming the outline of the beam or cornice are to be constructed of —— Ceco cold rolled channels in accordance with details furnished by the architect and spaced 16" c-c; 4" round steel pencil rods or ——Ceco cold rolled channels are to be fastened to the brackets with 14 gauge galvanized tie wire, the size and spacing being dependent upon dimensions of the beam or cornice, and finished construction being capable of supporting a dead load of 60 lbs. at any point. Ceco Metal Lath is then applied, using 18 gauge galvanized wire.

Hollow and Solid Metal Lath Partitions

Hollow Partition: (single studding) Drill and plug floor and ceiling to apply channel shoe and cap to which are fastened 16 or 18 gauge Ceco plain or perforated cold rolled channels, 2" to 31/2" in width at 131/2" c-c, lathing both sides with -- gauge Ceco Metal Lath, painted, using 18 gauge galvanized NOTE: Ceco Prong Studding, in 2" wire. NOTE: Ceco Prong Studding, in 2" to 6" widths, may be substituted, thereby eliminating the wiring of lath to studding. (Double Studding) Drill and plug floor and ceiling as before, using a double row of 34" or 1" Ceco plain cold rolled channels (1" channels for partition heights exceeding 14'-0"), spacing channels at such a distance as will give the required thickness of finished partition, studding to be braced at the midpoint between floor and ceiling with 34" or larger (depending upon thickness of partition) Ceco cold rolled channel securely wired to studding with 14 gauge galvanized wire, and Ceco Metal Lath to be applied to both sides as before.

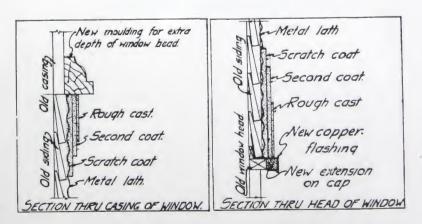


Solid Partition: Cut floor and ceiling to receive Ceco 16 gauge ¾" cold rolled channels, spaced $13\frac{1}{2}$ " c-c for partition heights up to 12'0" and 1" channels for greater heights, lathing one side only with — gauge Ceco Metal Lath, painted, using 18 gauge soft galvanized wire. The ¾" Tee Ceco Prong Stud may be used instead of plain channels. During construction, all studding is to be braced by temporary supports between floor and ceiling, until after the scratch coat of plaster has been applied.



Stucco Wall

Wood studs are to be spaced at 16" c-c and thoroughly braced in accordance with standard practice. Sheathing boards may be omitted and Ceco Self-Furring lath applied directly to the studding and back plastered. When Ceco Economy or Quality Lath is applied to studding or sheathing, Ceco Crimped Furring Strips must be used, being stapled with 1" 14 gauge staples every 12". the lath in turn being stapled every 8", and where laps occur tween supports, securely tied with 18 gauge galvanized wire. If no sheathing is used, inside If face of studding must be waterproofed with tar or asphalt. If sheathing is used, a high grade weather proof paper is to be applied to inside of wall.



Overcoating

The old weather boarding may be removed, and lath applied directly to the studding or sheathing, or if the weather boarding is in good condition, Ceco Self Furring Lath, or Ceco Economy or Quality Lath with Ceco Crimped Furring Strips, may be applied to the weather boarding. The old window and door frames should be extended to correspond with the new thickness of the walls. Stapling and wiring of lath to be carried on as shown under "Stucco Wall."



John Marshall School, Chicago, Illinois



Michigan Union Bldg., Ann Arbor Michigan



Sheridan Hall, Hays, Kansas



Rialto Theatre, Omaha, Nebraska



Clifton Hill School, Omaha, Nebraska

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Representative Buildings in which our Construction and Materials Have Been Used

SCHOOL BUILDINGS

Building

Los Feliz School
Anne Street School
Owensmouth High School
Micheltorena School
14th Street Intermediate School
Staunton Avenue School
Jefferson Polytechnic High School
Watsonville High School
Cogswell School
Administration Building for
University of Utah
Douglas School

Addition to Blaine School
High School
High School
High School
Bancroft School
State Normal School

Creighton University Gymnasium High School Building for Omaha University

Gymnasium
Bancroft School
School

High School
Yates School
Clifton Hill School
Druid Hill School
Park School
Field Club School
Train School
Junior High School
High School

High School
Grade Schools
Kearnes School
Milton Moore School
Bryant School
Lowell School
Central High School
Sheridan Hall

High School
Grade Schools
High School
Liberty High School
Dormitory A. & M. College
High School Addition
School Building for Juvenile

Training School
John Marshall School
Garfield School
Nurses School
Nurses Lodge
Michigan Union Building
Jackson Street School
Corlette School
Cleveland Heights School
Willoughby High School
Central School Annex
Western Reserve Dental College
East Technical High School
Science and Agriculture Building

Home Economics Building

Location

Los Angeles, Calif. Los Angeles, Calif. Owensmouth, Calif. Los Angeles, Calif. Los Angeles, Calif. Los Angeles, Calif. Los Angeles, Calif. Watsonville, Calif. San Francisco, Calif.

Salt Lake City, Utah Salt Lake City, Utah Salt Lake City, Utah Pocatello, Idaho Fremont, Nebr. Lincoln, Nebr. Lincoln, Nebr. Chadron, Nebr. Omaha, Nebr. Cherokee, Iowa Omaha, Nebr. Kearney, Nebr. Omaha, Nebr. Irwin, Iowa Havelock, Nebr. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Hastings, Nebr. Plattsmouth, Nebr. York, Nebr. Aurora, Nebr. Kansas City, Mo. Kansas City, Mo. Kansas City, Mo. Coffeyville, Kans. Coffeyville, Kans.
Tulsa, Okla.
Hays, Kans.
Marlin, Texas
Moberly, Mo.
Dewey, Okla.
Hutchinson, Kans.
College Station, Texas Moberly, Mo.

Gatesville, Texas
Chicago, Ill.
Garfield, Kans.
Milwaukee, Wis.
Muskegon, Mich.
Ann Arbor, Mich.
Canton, Ohio
Cleveland, Ohio
Cleveland, Ohio
Willoughby, Ohio
Cleveland, Ohio
Cleveland, Ohio
Cleveland, Ohio
Cleveland, Ohio
Cleveland, Ohio
Cleveland, Ohio
Columbus, Ohio

Architect or Engineer

City Architect
City Architect
H. H. Hewitt, Architect
City Architect
C. H. Russell, Architect
City Architect
City Architect
W. H. Weeks, Architect
Frederick Meyer, Architect

Cannon & Fatzer, Architects
Francis D. Rutherford, Architect
Cannon & Fatzer, Architects
F. H. Paradice, Jr., Architect
A. H. Dyer Co., Architects
Berlinghof & Davis, Architects
Berlinghof & Davis, Architects
Berlinghof & Davis, Architects
James C. Stitt, Architect
J. M. Nachtigall, Architect
Proudfoot, Bird & Rawson, Archts.
John & Alan McDonald, Architects
J. H. Craddock Co., Architects
J. H. Craddock Co., Architects
W. F. Gernandt, Architect
W. F. Gernandt, Architect
W. F. Gernandt, Architect
Thom & Alan McDonald, Architects
F. A. Henninger, Architect
George B. Prinz, Architect
Charles W. Steinbaugh, Architect
C. W. Way Co., Architects
George B. Berlinghof, Architect
George B. Berlinghof, Architect
C. W. Way Co., Architects
Smith, Rea & Lovitt, Architects
C. A. Henderson, Architect
C. A. Chandler, Kansas State Archt
Fonzie E. Robertson, Architect
Hawk & Parr, Architects
W. E. Hulse & Co., Architects
Architectural Department of College
W. H. Sayler, Architect

W. H. Clarkson, Architect
A. F. Hussander, Architect
W. E. Hulse & Co., Architects
Schuchardt & Judell, Architects
H. H. Weenhoff, Architect
Pond & Pond, Architect
George B. Hammond, Architect
Architect for Board of Education
Franz C. Warner, Architect
Franz C. Warner, Architect
Architect for Board of Education
Franz C. Warner, Architect
Architect for Board of Education
Franz C. Warner, Architect
Architect for Board of Education
Howard & Merrian, Architects
Joseph N. Bradford, Architect

HOSPITALS

San Francisco Emergency Hospital San Francisco City & County

Hospital
Mt. Zion Hospital
Addition to Latter Day Saints
Hospital
Orthopedic Hospital
Ford Hospital
State Hospital for Insane
Methodist Hospital
Atlantic Hospital
State Asylum for Feeble Minded
St. Anthony's Hospital Addition
Women's Hospital
Columbia Hospital
Children's Mercy Hospital
Henry Ford Hospital

San Francisco, Calif. San Francisco, Calif. San Francisco, Calif.

Salt Lake City, Utah Lincoln, Nebr. Omaha, Nebr. Norfolk, Nebr. Omaha, Nebr. Atlantic, Iowa Glenwood, Iowa Denver, Colo. Saginaw, Mich. Milwaukee, Wis. Kansas City, Mo. Detroit, Mich. John Reid, Jr., Architect

Herman Barth, Architect G. Albert Lansburgh, Architect

Pope & Burton, Architects
Burd F. Miller, Architect
J. T. Allan, Architect
James C. Stitt, Architect
George B. Prinz, Architect
Lloyd Willis, Architect
Henry F. Liebbe, Architect
F. W. Paroth, Architect
Cowles & Mutscheller, Architects
Schmidt, Garden & Martin, Archts.
Wight & Wight, Architects
Architectural Department of
Hospital

Page Twenty-nine



Security Mutual Building, Lincoln, Nebraska



Omaha Grain Exchange, Omaha, Nebraska



Michigan State Telephone Co. Bldg., Detroit, Michigan



Wieboldt Department Store, Chicago, Illinois



Miller & Paine Stores, Lincoln, Nebraska



Exchange National Bank, Tulsa, Oklahoma

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OFFICE AND STORE BUILDINGS

Building

Building
Merchants National Bank Building
Kerekhoff Building
Kerekhoff Building
M. J. Connell Building
M. J. Connell Building
Southern Title Guaranty Building
Olender Building
Grangers Building
Twohy Building
Addition to Standard Oil Building
Addition to Fife Building
Addition to Emporium Building
Northwestern Pacific Building
McCreery Estate Building
Colonel Hudson Building
First National Bank Building
Firestone Tire & Rubber Company
Building

Firestone Tire & Rubber Company
Building
Omaha Grain Exchange Building
Securities Building
Miller & Paine Store Buildings
Security Mutual Life Building
Rudge & Guenzel Store Building
Oil Exchange Building
Hynds Building
Francis Building
Martin Building
Park Store Building
Marten Store Building
Marten Store Building
Flannigan-Sewage Realty Company
Building

Marten Store Building
Flannigan-Sewage Realty Company
Building
Ridge Arcade Building
Firestone Tire & Rubber Company
Building
John Doherty Building
Railway Exchange Building
Tharp Wallace Building
Exchange National Bank
Gustin Bacon Service Station
Wittman Building
Wirthman Store, Office and Theatre
Building
Oklahoma Producing & Refining
Company Building
Freeport State Bank
Weiboldt Department Store
Michigan State Telephone Company
Building
Rayner-Dalheim Building
Laird-Norton Building
May Building
May Building
Marshall Building
Heller Brothers Building
Baruch Mahler Commercial Building

Baruch Mahler Commercial Building Skeel Hall

Location

Los Angeles, Calif.
Los Angeles, Calif.
Los Angeles, Calif.
Los Angeles, Calif.
San Diego, Calif.
Fresno, Calif.
Hollister, Calif.
San Jose, Calif.
San Francisco, Calif.
Ogden, Utah
Pocatello, Idaho Ogden, Utah Pocatello, Idaho

Omaha, Nebr.
Omaha, Nebr.
Omaha, Nebr.
Lincoln, Nebr.
Lincoln, Nebr.
Lincoln, Nebr.
Casper, Wyo.
Cheyenne, Wyo.
Des Moines, Iowa
Storm Lake, Iowa
Storm Lake, Iowa

Kansas City, Mo. Kansas City, Mo.

Kansas City, Mo. Kansas City, Mo. Kansas City, Mo. Blackwell, Okla. Tulsa, Okla Kansas City, Mo. Kansas City, Mo.

Kansas City, Mo.

Tulsa, Okla. Rock Island, Ill. Freeport, Ill. Chicago, Ill.

Detroit, Mich. Chicago, 11l. Winona, Minn. Cleveland, Ohio Cleveland, Ohio Cleveland, Ohio

Cleveland, Ohio Cleveland, Ohio

Architect or Engineer

Architect or Engineer
William Curlette & Son, Architects
Morgan, Walls & Morgan, Architects
Albert Martin, Architect
George W. Harding, Engineer
Theodore C. Kistner, Architest
Eugene Mathewson, Architect
William Binder, Architect
William Binder, Architect
William Binder, Architect
P. J. Walker Co., Architects
Sylvain Schnaittacher, Architect
Morris M. Bruce, Architect
O'Brien Bros., Architects
Willis Polk Co., Architects
Shreeve & Madsen, Architects

John Latenser & Sons, Architects F. A. Henninger, Architect F. A. Henninger, Architect Berlinghof & Davis, Architects Berlinghof & Davis, Architects G. H. Ellsworth, Architect Garbutt & Weidner, Architects William Dubois, Architects William Dubois, Architects Sawyer & Watrous, Architects Beuttler & Arnold, Architects Marten & Sutherland, Architects Marten & Sutherland, Architects

Smith, Rea & Lovitt, Architects Smith, Rea & Lovitt, Architects

Smith, Rea & Lovitt, Architects E. P. Madorie, Architect Wight & Wight, Architects Crowell & Van Meter, Architects Weary & Alford, Architects Smith, Rea & Lovitt, Architects Smith, Rea & Lovitt, Architects

Smith, Rea & Lovitt, Architects

C. K. Birdsall, Architect Frank A. Carpenter, Architect Frank A. Carpenter, Architect R. C. Berlin, Architect

Smith, Hinchman & Grylls, Archts. T. R. Bishop, Architect
Schmidt, Garden & Martin, Archts. Graham Burnham & Co., Architects
W. S. Lougee, Architect
Christian Schwarzenberg & Gaede, Architects
Richardson & Yost, Architects
Frank D. Skeel, Architect

HOTELS AND APARTMENTS

Morshead Apartment Wilson Apartment Alice Apartments Castle Hotel Morris Apartments Clarke Hotel Drake Apartments (eight buildings)

St. Regis Apartments

Blackstone Hotel

Elwood Apartments

Hotel Conant Coronado Apartments

Kingsborough Apartments

Hotel

Hotel

Home Builders Apartments Hotel Hotel
Midwest Hotel
Oldham Hotel
Garden Apartments
Bancroft Hotel
Parkway Apartment Smith Apartments Swanson Apartment Dr. G. W. Crile Hotel

San Francisco, Calif. San Francisco, Calif. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Hastings, Nebr.

Omaha, Nebr.

Omaha, Nebr.

Omaha, Nebr.

Omaha, Nebr. Omaha, Nebr.

Omaha, Nebr. Grand Island, Nebr.

Kearney, Nebr.

Scottsbluff, Nebr.

Omaha, Nebr. York, Nebr. Kansas City, Mo. Kansas City, Mo. Chicago, Ill. Saginaw, Mich. Chicago, Ill. Chicago, Ill. Chicago, Ill. Cleveland, Ohio

Houghton Sawyer, Architect
C. A. Meussdorffer, Architect
F. A. Henninger, Architect
John McDonald, Architect
James T. Allan, Architects
C. W. Way Co., Architects
Drake Realty Construction Co.,
Architects
Bankers Realty Investment Co.,
Architects
Bankers Realty Investment Co.,
Architects
Drake Realty Construction Co.,
Architects
John & Alan McDonald, Architects
Drake Realty Construction Co.,
Architects
John & Alan McDonald, Architects
Drake Realty Construction Co.,
Architects
Fred Nelson, Architect
Bankers Realty Investment Co.,
Architects
Sankers Realty Investment Co.,
Architects
Smith, Rea & Lovitt, Architects
Smith, Rea & Lovitt, Architects
Schmidt, Garden & Martin, Archts.
Schmidt, Garden & Martin, Archts.
R. C. Berlin, Architect
Frank D. Skeel, Architect



U. S. Garage, San Francisco, California



Hudson Stuyvesant Building, Cleveland, Ohio



Rayner-Dalheim Bldg., Chicago, Illinois



Sample-Hart Garage, Omaha, Nebraska



Lee-Coit-Andreeson Co. Warehouse, Omaha, Nebraska



Firestone Tire & Rubber Co. Building, Kansas City, Missouri

GARAGES

Building

Building
Babbitt Garage
Liberty Garage
Bigelow Garage
St. George Garage
Powell Street Garage
Podesta Garage
Hiatt Realty Company
Service Garage
West Farnam Garage
Sample Hart Garage
Scott Garage
Blacktone Garage

Sunderland Bros. Garage
Freeman Garage
Smith-Dorsey Building
Strode Garage
Du Teil Garage
Zimmerer Garage
Kasparek Garage
Kasparek Garage
Kerr Estate Garage
Brandeis Garage
T H. Pollock Co. Garage
Mills County Garage
Romans Garage
Ficke Garage
Iles & Weir Garage
Iles & Weir Garage
McQueenie Garage
McQueenie Garage
McQueenie Garage
McAuto Sales Building
Carhart Motor Company Building
McClelland-Gentry Motor Company
Building Sunderland Bros. Garage

McClelland-Gentry Motor Com Building Mathis Garage Skeel Bros. Garage Hudson Stuyvesant Building Adams Oakland Building Building for Cooke Realty & Investment Co. Boyer Bros. Building

Diamond Laundry Co Building
Crescent Creamery Co.
Douglas Printing Co. Building
Shafer Printing Co. Building
Gordon-Lawless Building
Star Van & Storage Co. Building
Petersen-Pegau Bakery
Graham Ice Cream Co. Office and
Factory
Lee-Coit-Andreesen Co. Warehouse
Douglas Motors Corporation, Office
and Factory
Firestone Tire & Rubber Co Bldg.
Cook Paint & Varnish Co. Bldg.
Williamson Halsell Frasier Co.
Warehouse
Liberty Dairy Building
C. & N W. Calumet Elevator Bldg.
John Becker Building
Van Dorn Electric Co. Building
Atlas Car & Manufacturing Co. Bldg.

Municipal Pier
Memorial Art Gallery
Omaha Athletic Club
Old Peoples Home
House of Good Sheperd
Rialto Theatre
Des Moines Municipal Court House
Dodge County Court House
Clay County Court House
Masonic Temple
Auditorium
Swope Park Music Pavilion
Nettleton Home
Pawnee County Court House
Kansas Masonic Home
Elks Club

Fleanor Club
Dairy Barn Building
Girl's Dormitory
Printing Building
Dormitory 54
Dormitory 55
East Legion Hall
Esther J. Davis Hall
Industrial Building
Ladies of Maccabees Building
H. H. Timken Residence
Corning Residence Corning Residence

Location

Location
Flagstaff, Ariz.
San Francisco, Calif.
Omaha, Nebr.

Omaha, Nebr.

Omaha, Nebr.
Lincoln, Nebr.
Lincoln, Nebr.
Lincoln, Nebr.
Lincoln, Nebr.
Seward, Nebr.
Davis City, Nebr.
Hastings, Nebr.
Hastings, Nebr.
Plattsmouth, Nebr.
Glenwood, Iowa
Denison, Iowa
Davenport, Iowa
Davenport, Iowa
Kansas City, Mo.
Kansas City, Mo.
Oklahoma City, Okla.

Oklahoma City, Okla. Tampico, Ill Cleveland, Ohio. Cleveland, Ohio Cleveland, Ohio

Cleveland, Ohio Akron, Ohio

Architect or Engineer

Architect or Engineer

George W. Harding, Engineer
T. Patterson Ross, Architect
August Headman, Architect
O'Brien Brothers, Architects
W. H. Toepke, Architect
Perseo Righetti, Architect
John McDonald, Architect
James T. Allan, Architect
James T. Allan, Architect
James T. Allan, Architect
Geo. B. Prinz, Architect
James T. Allan, Architect
F. A. Henninger, Architect
Bankers Realty Investment Co.,
Architects
John & Alan McDonald, Architects
C. H. Larsen, Architect
C. H. Larsen, Architect
Grabe & Meginnis, Architects
Grabe & Helleberg, Architects
R. A. Bradley & Co., Architects
R. A. Bradley & Co., Architect
F. A. Henninger, Architect
George A. Berlinghof, Architect
J. Chris Jensen, Architect
Clausen & Kruse, Architects
Clausen & Kruse, Architects
Clausen & Kruse, Architect
S. R. Frink, Architect
Smith, Rea & Lovitt, Architects
Layton & Smith, Architects
Clausen & Kruse, Architects
Clausen & Kruse, Architects
Chayton & Smith, Architects
Clausen & Kruse, Architects
Clausen & Kruse, Architects
Clausen & Kruse, Architects
Chayton & Smith, Architects
Clausen & Kruse, Architects

Layton & Smith, Architects Clausen & Kruse, Architects Frank D. Skeel, Architect Skeel Bros, Architects The Building Service Co., Architects

Lehman & Schmitt, Architects Boyer Bros., Architects

WAREHOUSES AND FACTORIES

Los Angeles, Calif. Los Angeles, Calif. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Lincoln, Nebr. Omaha, Nebr.

Omaha, Nebr. Omaha, Nebr.

Omaha, Nebr. Kansas City, Mo. Kansas City, Mo. Wichita, Kans.

Oklahoma City, Okla. Chicago, Ill. Chicago, Ill. Cleveland, Ohio Cleveland, Ohio Cleveland, Ohio

Train & Williams, Architects
George W. Harding, Engineer
James T. Allan, Architect
John & Alan McDonald, Architects
Lloyd Willis, Architect
C. H. Larsen, Architect
C. D. Cooley Co., Architects

Richard Everett, Architect Henry Raapke, Architect

O. H. Strauser Co., Contractors Smith, Rea & Lovitt, Architects Smith, Rea & Lovitt, Architects Smith, Rea & Lovitt, Architects

Layton & Smith, Architects
A. L. Himelblau, Architect
Witherspoon-Engler Co., Contractors
Charles E. Tousley, Architect
William J. Carter, Architect
W. S. Ferguson Co., Architects

OTHER BUILDINGS

Redondo Beach, Ca Palo Alto, Calif. Omaha, Nebr. Omaha, Nebr. Omaha, Nebr. Des Moines. Iowa Fremont, Nebr. Lincoln, Nebr. Kearney, Nebr. Kansas City, Mo. Kansas City, Mo. Larned, Kans. Wichita, Kans. Chicago, III. Redondo Beach, Calif.

Chicago, Ill. Mooseheart, Ill Mooseheart, Ill Mooseheart, III. Mooseheart, III. Mooseheart, III. Mooseheart, III. Mooseheart, III. Mooseheart, III. Port Huron, Mich. Canton, Ohio Cleveland, Ohio George W. Harding, Engineer
Bakewell & Brown, Architects
John Latenser & Sons, Architects
John McDonald, Architect
John Latenser & Sons, Architects
Associate Architects of Des Moines
A. H. Dyer Co., Architects
W. F. Gernandt, Architects
Berlinghof & Davis, Achitects
Gerlinghof & Davis, Achitects
J. H. Craddock Co., Architects
City Hall Architect
Wight & Wight, Architects
W. E. Hulse & Co., Architects
Edward L. Tilton, Architects
Edward L. Tilton, Architect
Edward L. Tilton, Architect
Schmidt, Garden & Martin, Archts.
Robert F. Havlik, Architect
Robert F. Havlik, Archit



Blackstone Hotel, Omaha, Nebraska



Garden Apartments, Chicago, Illinois



Oldham Hotel, Kansas City, Missouri



Smith Apartments, Chicago, Illinois



Hotel, Grand Island, Nebraska



Swanson Apartments, Chicago, Illinois

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Elks Club, Chicago, Illinois



Parkway Apartments, Chicago, Illinois



Nettleton Home, Kansas City, Missouri



Freeport State Bank, Freeport, Illinois



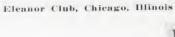
Rice Building, Boston Massachusetts



Municipal Pier, Redondo Beach, California



Mercy Hospital, Kansas City, Missouri





Athletic Club, Omaha, Nebraska



San. Francisco Hospital, San Francisco, California



Henry Ford Hospital, Detroit, Michigan



Morshead Apartments, San Francisco, California

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Christiansen Bldg., Chicago, Illinois



Fort Shelby Hotel, Detroit, Michigan



Columbia Hospital, Milwaukee, Wisconsin



Dodge County Court House, Fremont, Nebraska



Conant Hotel, Omaha, Nebraska



14th Street School, Los Angeles, California

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